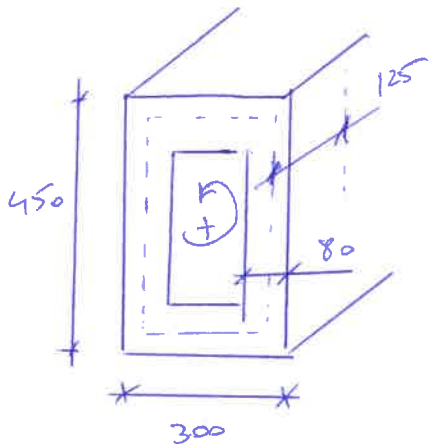


Question

(i)



$$A_s = \frac{\pi \cdot b^2}{4} = 78,5 \text{ mm}^2$$

$$f_{yd} = 435 \text{ MPa}$$

$$A_e = (300 - 80) \cdot (450 - 80) = 81400 \text{ mm}^2$$

$$u = (300 - 80) \times 2 + (450 - 80) \times 2 = 1180 \text{ mm}$$

$$q = \frac{T}{2 A_e} = \frac{60 \cdot 10^6}{2 \cdot 81400} = 368,55 \text{ N/mm}$$

$$\text{and } q = f_{yd} \cdot \sqrt{\frac{A_w \sum A_e}{s \cdot u}}$$

$$\begin{aligned} \text{so } \sum A_e &= \left(\frac{q}{f_{yd}} \right)^2 \cdot \frac{s \cdot u}{A_w} \\ &= \left(\frac{368,55}{435} \right)^2 \cdot \frac{125 \cdot 1180}{78,5} \\ &= \underline{\underline{1617,70 \text{ mm}^2}} \end{aligned}$$

$$(ii) \quad \tan^2 \theta = \frac{A_w \cdot u}{s \cdot \sum A_e} = \frac{78,5 \cdot 1180}{125 \cdot 1617,70} = 0,382$$

$$\Rightarrow \theta = 31,72^\circ$$

$$q = \sigma_c \cdot t \cdot \sin \theta \cos \theta$$

$$\sigma_c = \frac{q}{t \sin \theta \cos \theta} = \frac{368,55}{80 \cdot \sin 31,7 \cos 31,7} = 10,30 \text{ MPa}$$

$$\sigma_c < \frac{V \cdot f_{cd}}{0,5} \Rightarrow f_{cd} = \frac{\sigma_c}{0,5} = \underline{\underline{20,60 \text{ MPa}}}$$

(iii)

$V_{Rd,s}$?

assume $\cot \theta = 2,5$

stirrups are 2 legged : $A_{sw} = 2 \cdot 78,5 = 157,1 \text{ mm}^2$
 $f_{ydw} = 435 \text{ MPa}$

$$\begin{aligned} V_{Rd,s} &= \frac{A_{sw}}{s} \cdot z \cdot f_{ydw} \cot \theta \\ &= \frac{157,1}{125} \cdot 0,9 \cdot (450 - 40) \cdot 435 \cdot 2,5 \\ &= \underline{\underline{420,2 \text{ kN}}} \end{aligned}$$

check compressive stress :

$$\alpha_{cw} = 1$$

$$\eta_1 = 0,5$$

$$\begin{aligned} V_{Rd,max} &= \alpha_{cw} \cdot b_w \cdot z \cdot \eta_1 \cdot f_{cd} / (\cot \theta + \tan \theta) \\ &= 1 \cdot 300 \cdot 0,9 \cdot (450 - 40) \cdot 0,5 \cdot 20,60 \\ &\quad \underline{\underline{\cot \theta + \tan \theta}} \\ &= \underline{\underline{436,9 \text{ kN}}} > 420,2 \text{ kN} \text{ so } \cot \theta = 2,5 \\ &\quad \text{is OK } \checkmark \end{aligned}$$

(iv)

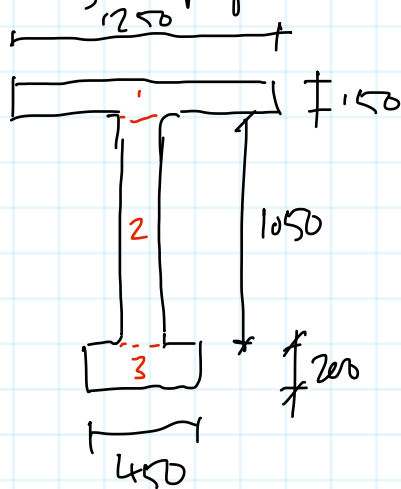
$$\frac{T_{Ed}}{T_{Rd,max}} + \frac{V_{Ed}}{V_{Rd,max}} \leq 1$$

with $T_{Ed} = 30 \text{ kNm}$
 $T_{Rd,max} = 60 \text{ kNm}$
 $V_{Rd,max} = 420,2 \text{ kN}$

$$\begin{aligned} \Rightarrow V_{Ed} &= \left(1 - \frac{T_{Ed}}{T_{Rd,max}} \right) \cdot V_{Rd,max} \\ &= \underline{\underline{210,1 \text{ kN}}} \end{aligned}$$



Q2 (a) Any simplification acceptable. e.g:



$$\begin{aligned} A_1 &= 187500 \\ A_2 &= 168000 \\ A_3 &= 90000 \\ \hline &= 445500 \end{aligned}$$

$$\bar{y} = \frac{\sum Ay}{\sum A} = 851 \text{ mm}$$

$$\begin{aligned} I &= \sum I_{\text{local}} + A\bar{y}^2 \quad (\text{full calc not shown}) \\ &= 11.6 \times 10^9 \text{ mm}^4 \end{aligned}$$

(b) $P = 900 \times 3000 = 2.7 \text{ MN}$

$$P/A = \frac{2.7 \times 10^6}{445500} = 6.06 \text{ MPa}$$

$$e = 851 - 125 = 726 \text{ mm}$$

$$z_1 \therefore \frac{I}{y_1} = \frac{11.6 \times 10^9}{(1400 - 851)} = 2.03 \times 10^8 \quad (\text{top})$$

$$z_2 = \frac{I}{y_2} = \frac{11.6 \times 10^9}{851} = 1.31 \times 10^8 \quad (\text{bottom})$$

$$\frac{P \cdot e}{z_1} = \frac{2.7 \times 10^6 \times 726}{z_1} = -9.63 \text{ MPa}$$

$$\frac{P \cdot e}{z_2} = \frac{2.7 \times 10^6 \times 726}{z_2} = 14.09 \text{ MPa}$$



(b) cont.

$$M_{min} = 600 \text{ kNm}$$

$$M_{max} = 2850 \text{ kNm}$$

Using data book equations, we get:

	Top	Botm
σ_{max}	10.43	-0.72
σ_{min}	-0.63	16.43

(all MPa)

(c) Choose prestressing force, e.g. $\frac{f_c}{2} A = \underline{\underline{4MN}}$
use data book equations to find limits on 'e'.

Top fibre:

$$\frac{f_c z_1}{\rho} - \frac{z_1}{\rho} + \frac{M}{\rho} < e < \frac{f_t z_1}{\rho} - \frac{z_1}{A} + \frac{M}{\rho}$$

Bottom fibre:

$$\frac{f_c z_2}{\rho} - \frac{z_2}{A} + \frac{M}{\rho} > e > \frac{f_t z_2}{\rho} - \frac{z_2}{A} + \frac{M}{\rho}$$

we can then find, for this simplification:

$$\text{top: } 254 \text{ mm} < e < 657 \text{ mm}$$

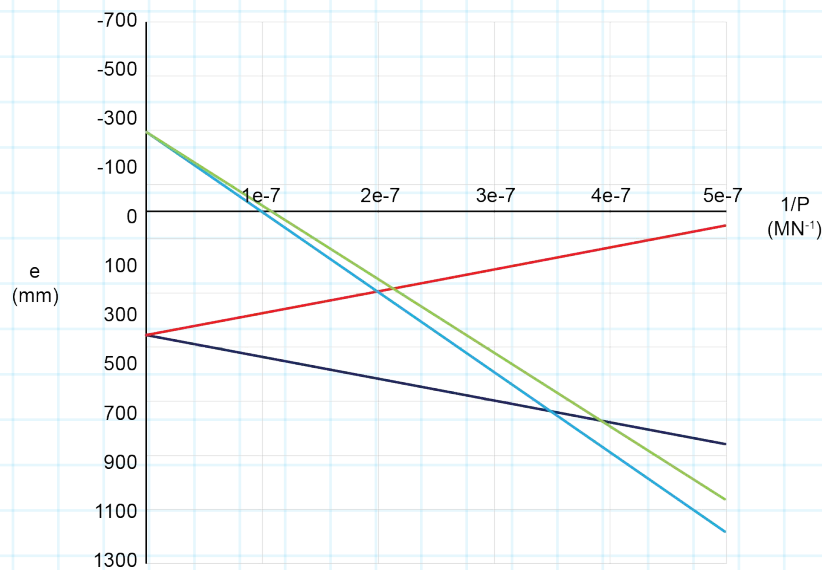
$$\text{bottom: } 384 \text{ mm} < e < 444 \text{ mm}$$

And using equation allows us to plot the Moyal diagram.



(c)
Cont.

Magnel diagram:



(d) To find P_{max} , set relevant equations equal

$$\Rightarrow P_{max} = \underline{\underline{5023 \text{ kN}}} \quad (e = 295 \text{ mm})$$

& for P_{min} , the same:

$$\Rightarrow P_{min} = \underline{\underline{2550 \text{ kN}}} \quad (e = 772 \text{ mm})$$

(e) Section as proposed close to P_{min} .



Q3(a)

Cover = 75mm

@ 60 years average carbonation = 18mm, S.D. = 4mm

for average: $\frac{17 \text{ mm}}{75 \text{ mm}} \propto \sqrt{t}$

$$\frac{75}{17} = \sqrt{\frac{t}{60}} \Rightarrow t = 129 \text{ years}$$

$\therefore$ 69 years remain.

Check upper limit at say 95% confidence

$$x = 17 + 1.645(2) = 20.3 \text{ mm}$$

$$\frac{75}{20.3} = \sqrt{\frac{t}{60}} \Rightarrow t = 91 \text{ years}$$

$\therefore$ 31 years remain

$\therefore$ ambition may be achievable, but may require coatings etc. to reduce carbonation in places

i.e. a cautious approach is warranted.

(b)

- Chloride ingress
- freeze thaw
- ASR reaction
- sulfate attack
- quality & permeability of original concrete
- existing cracks & repairs.
- inspection & maintenance regime

etc..



- Q3(c)
- structural capacity & code compliance
 - review available drawings
 - SLS & ULS checks
 - fire safety
 - seismic capacity.
 - repair & protection
 - spalling / cracks
 - patch repairs to carbonated areas
 - corrosion protection / mitigation / inhibitors
 - monitoring & maintenance plan
 - inspection schedule
 - inspection of protective systems
 - STM
 - integrating old & new
 - load paths & connections
 - foundation / settlement impact
 - creep / shrinkage
 - dynamics & seismic
 - performance testing
 - load test (eg proof load)
 - material testing
 - seismic assessment
 - whole life carbon assessment
 - including plans to replace parts
 - documentation
 - calculation to show benefit of retention.

etc.